CD'S

CALIFORNIA DEPARTMENT OF PARKS AND RECREATION
ACQUISITION AND DEVELOPMENT DIVISION

SOUTH YUBA RIVER STATE PARK

HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

(National Register of Historic Places: No. 71000168), (California Historic Landmark: No. 390), (Historic American Engineering Record: No. CA-41), (Historic American Building Survey: No. CA-1404)

CSFM # 01-29-11-0032

1-ALL MATERIALS SHOWN OR NOTED ON THE PLANS ARE NEW UNLESS CALLED OUT OTHERWISE.

2-THE CONTRACTOR SHALL VISIT THE SITE AND VERIFY ALL EXISTING CONDITIONS SHOWN OR DIMENSIONED HERE. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STATE REPRESENTATIVE FOR RESOLUTION BEFORE PROCEEDING WITH THAT PORTION OF THE WORK.

3-ALL WORK SHALL COMPLY WITH THE CURRENT EDITION OF THE FOLLOWING LISTED CODES, AND ALL OTHERS HAVING JURISDICTION OVER THE WORK.
TITLE 19, CCR, PUBLIC SAFETY, SFM REGULATIONS.
2016 CA ADMINISTRATIVE CODE TITLE 24, PT 1.
2016 CA BUILDLING CODE (CBC) TITLE 24, PT 2.
2016 CA ELECTRICAL CODE (CEC) TITLE 24, PT 3.
2016 CA MECHANICAL CODE (CMC) TITLE 24, PT 4
2016 CA PLUMBING CODE (CPC) TITLE 24, PT 5.
2016 CA ENERGY CODE CCR TITLE 24, PT 6.
2016 CA GREEN BUILDING STANDARDS TITLE 24, PT 11.
2016 CA REFERENCED STANDARDS TITLE 24, PT 12.
2016 CA BUILDING STANDARDS ADMINISTRATIVE CODE; CCR TITLE 19, PUBLIC SAFETY: DIVISION 1. STATE FIRE MARSHAL; AND CCR TITLE 24 ADA REQUIREMENTS.
2010 ADA STANDARD FOR ACCESSIBLE DESIGN.
THE SECRETARY OF THE INTERIOR'S STANDARDS FOR THE TREATMENT OF HISTORIC PROPERTIES.

4-CONDUCT ALL WORK IN ACCORDANCE WITH THE LATEST SAFETY RULES AND REGULATIONS OF ALL AUTHORITIES AND AGENCIES HAVING JURISDICTION OVER THE WORK.

5-ALL WORK SHALL BE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. WHERE DETAILED INFORMATION OR CLARIFICATION IS REQUIRED, THE MATTER SHALL BE REFERRED TO THE STATE REPRESENTATIVE FOR WRITTEN RESOLUTION.

6-THE CONTRACTOR SHALL NOT SCALE THE DRAWINGS, BUT SHALL RELY ONLY ON THE WRITTEN DIMENSIONS GIVEN. IF A DISCREPANCY OCCURS OR NO DIMENSION IS GIVEN, THE CONTRACTOR SHALL NOTIFY THE STATE REPRESENTATIVE FOR WRITTEN CLARIFICATION BEFORE PROCEEDING WITH THAT PORTION OF THE WORK.

7-THE CONTRACTOR SHALL FIELD VERIFY DIMENSIONS AND EXISTING CONDITIONS AT THE SITE AND SHALL REPORT ANY DISCREPANCIES IN WRING TO THE STATE REPRESENTATIVE PRIOR TO SUBMITTALS

8-ALL DIMENSIONS ARE GIVEN FROM THE FACE OF CONCRETE, CONCRETE MASONRY UNITS, STRUCTURAL STEEL, STUD OR CENTERLINE OF PARTITION UNESS OTHERWISE NOTED

GENERAL NOTES

1.	THIS IS A REGISTERED HISTORICAL PLACE AND STRUCTURE.	
2.	OCCUPANCY CLASSIFICATION AND USE	U
3.	BUILDING CONSTRUCTION TYPE	V-B
4.	NUMBER OF STORIES	1
5.	ACTUAL BUILDING HEIGHT	28'6"
6.	BUILDING AREA IN SQUARE FEET	2470
7.	AREA OF PROJECT IN SQUARE FEET	NO CHANGE
8.	ALLOWABLE AREA PER C.B.C.	5500
9.	FIRE ALARM	NO
	TYPE: (MANUAL, AUTOMATIC, ETC)	
10.	OTHER FIRE PROTECTION SYSTEMS, IF ANY	HYDRANT
11.	SMOKE CONTROL SYSTEM	NO
12.	OCCUPANT LOAD (Posted based on struct capacity)	50
13.	YEAR BUILDING WAS CONSTRUCTED	1862
14.	IN A HIGH FIRE HAZARD SEVERITY ZONE?	YES

BUILDING CODE REVIEW

A / A1 — SECTION NUMBER / SHEET NUMBER

1 / A1 — DETAIL NUMBER / SHEET NUMBER

1 — REVISION NUMBER

SYMBOLS LEGEND

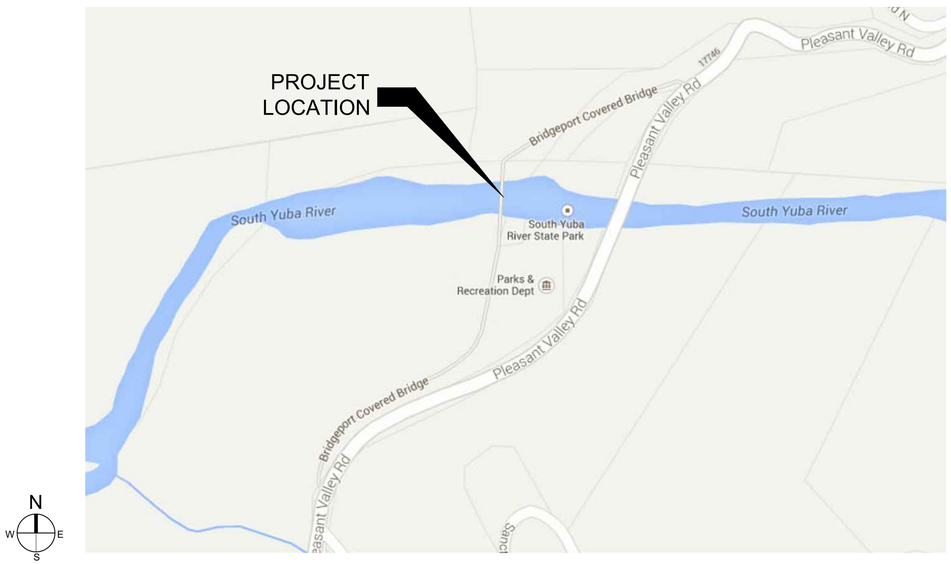
- (N) STABILIZED SOIL
- CONCRETE BLOCK
- CONCRETE
- PLYWOOD
- WOOD FRAMING
- WOOD BLOCKING
- WOOD, FINISH
- METAL
- UNDISTURBED SOIL
- DISTURBED SOIL
- (N) CONCRETE PAVING
- (N) AC PAVING

MATERIAL LEGEND

REPAIR, RESTORE AND REHABILITATE, TO ITS ORIGINAL USE THE 208 FEET, SINGLE SPAN, HISTORIC TRUSS/ARCH, AND COVERED BRIDGE. WORK WILL INCLUDE REPLACING THE EXISTING TEMPORARY, NONE-HISTORIC SUPPORTS WITH PERMANENT BRIDGE MEMBERS AND SUPPORTS DESIGNED TO BE COMPATIBLE AND REPRESENTATIVE OF THE ORIGINAL DESIGN AND APPEARANCE.

SCOPE OF WORK

VICINITY MAP



LOCATION MAP

SHEET INDEX

OFFICE OF THE STATE FIRE MARSHAL
APPROVED FOR PLANS REVIEW ONLY
Reviewed by: David Nye, OSFM
JAN 09 2018
Approval of this plan does not authorize or approve any omission or deviation from applicable regulations. Final approval is subject to field inspection. One set of approved plans shall be available on the project site at all times.

For field inspections, contact:
Deputy Brad Goodrich at 916-445-9930
to schedule appointments

APPROVAL

CALIFORNIA STATE PARKS SINCE 1864

ACQUISITION & DEVELOPMENT DIVISION
One Capitol Mall
Sacramento, CA
95814-3229

LICENSED ARCHITECT GIL DE LA PENA
No. C21464
Ren 1.31.19
STATE OF CALIFORNIA Jan.2018

CALIFORNIA STATE FIRE MARSHAL- APPROVED

Approval of this plan does not authorize or approve any omission of deviation from applicable regulations. Final approval is subject to field inspection. One set of approved plans shall be available on the project site at all times.

Reviewed by _____ Date _____

DPR ACCESS COMPLIANCE REVIEW
ACCESSIBILITY SECTION
CERTIFICATION # 18-025
Shawn B. Smith, CASp-009
Reviewed by _____ Date 1/14/16

ACCESSIBILITY COMPLIANCE AND STATE FIRE MARSHAL SIGNED ORIGINALS ARE ON FILE AT THE DEPARTMENT OF PARKS AND RECREATION, NORTHERN SERVICE CENTER

DESIGNED:	DESIGNER
DRAWN:	STAFF
CHECKED:	SUPERVISOR
DATE:	DECEMBER 2017

REVISIONS	
	DATE

SOUTH YUBA RIVER STATE PARK
HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

COVER SHEET

DRAWING NO.
30419.001

SHEET NO.
G1.0
001 OF 030

File: Q:\South_Yuba_River_SP\202193-Covered_Bridge_Stabilization\10_Architectural_Working_Files\ArchDrawingFiles\Sheet Set\G1.0 COVER SHEET.dwg Layout: G1.0 COVER SHEET Date: February 02 2018 - 3:12 pm User: GDELA

CD'S

&	AND
(E)	EXISTING
(N)	NEW
@	AT
A/C	AIR CONDITIONING
AB	AGGREGATE BASE OR ANCHOR BOLTS
AC	ASPHALTIC CONCRETE
ACI	AMERICAN CONCRETE INSTITUTE
ACP	ASBESTOS CEMENT PIPE
ACST	ACOUSTIC
ADA	AMERICAN DISABILITIES ACT
ADJ	ADJUSTABLE
AFF	ABOVE FINISHED FLOOR
AGG	AGGREGATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ALUM	ALUMINUM
APA	AMERICAN PLYWOOD ASSN.
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
ARV	AIR RELEASE VALVE
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ASPH	ASPHALT
AV	AIR VALVE
AWS	AMERICAN WELDING SOCIETY
BOT	BOTTOM
B.	BOTTOM (REINFORCEMENT DRAWINGS)
BAB	BOTTOM ALTERNATE BENT
BC	BOTTOM OF CURB
BOW	BOTTOM OF WALL
BFP	BACKFLOW PREVENTER
BLDG	BUILDING
BM	BENCHMARK
BO	BLOW OFF VALVE
BOT	BOTTOM
BOL	BEGINNING OF LINE
BOS	BOTTOM OF STEP
BP	BASE PLATE
BRG	BEARING
BV	BALL VALVE
BVC	BEGIN VERTICAL CURVE
BW	BOTTOM OF WALL
C	CENTERLINE
CC	CENTER TO CENTER
CEM	CEMENT
CFS	CUBIC FEET PER SECOND
CG	CAMPGROUND
CG&S	CURB GUTTER & SIDEWALK
CHKR	CHECKER
CIP	CAST IRON PIPE
CJ	CONSTRUCTION JOINT
CL OR CLR	CLEAR
CMP	CORRUGATED METAL PIPE
CS	COMFORT STATION
CMU	CONCRETE MASONRY UNIT
CO	CLEANOUT
COLS	COLUMNS
COMB.	COMBINATION
CONC	CONCRETE
CONST.	CONSTRUCTION
CONT	CONTINUOUS
COSS	CLEAN OUT SANITARY SEWER
C.P.	CONTROL POINT
CS	COMFORT STATION
CSK	COUNTERSUNK
CSP	CALIFORNIA STATE PARKS
CV	CHECK VALVE
CT	CERAMIC TILE
CW	COLD WATER
D OR DIA	DIAMETER
DBH	DIAMETER AT BREAST HEIGHT
DET	DETAIL
DF	DRINKING FOUNTAIN
DG	DECOMPOSED GRANITE
DI	DROP INLET, DUCTILE IRON
DIAG	DIAGONAL
DIM	DIMENSION
DIST	DISTRIBUTION
DN	DOWN
DPR	DEPARTMENT OF PARKS AND RECREATION
DR	DOOR
DO	DRAIN OUTLET
DWG	DRAWING
DWR	DEPARTMENT OF WATER RESOURCES
EA	EACH
EC	END CURVE
EF	EACH FACE
EFEW	EACH FACE EACH WAY
EJ	EXPANSION JOINT
EL	ELEVATION
ELECT	ELECTRICAL
ELEV	ELEVATION
EOL	END OF LINE
EP	EDGE OF PAVEMENT
EPB	ELECTRIC PULL BOX
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
EVC	END VERTICAL CURVE
EW	EACH WAY
(E)	EXISTING
EO	EDGE OF
ES	EXISTING SURFACE
EXP	EXPANSION
FOF	FACE OF FINISH
FA	FLANGE ADAPTER
FB	FLAT BAR
FCA	FLANGED COUPLING ADAPTER
FD	FLOOR DRAIN
FG	FINISHED GRADE
FH	FIRE HYDRANT
FIN	FINISHED
FF	FINISHED FLOOR
FRP	FIBER REINFORCED PLASTIC
FS	FINISH SURFACE
FL	FLOW LINE
FL. OR FLR	FLOOR
FM	FORCE MAIN
FOF	FACE OF FINISH
FRMG	FRAMING
FT	FOOT OR FEET
FTG	FOOTING
GA	GAUGE
GAL	GALLON
GALV	GALVANIZED
GB	GRAB BAR
GD	GUTTER DRAIN
GFI	GROUND FAULT INTERRUPTER
GI	GALVANIZED IRON
GPM	GALLONS PER MINUTE
GS	GALVANIZED STEEL
GV	GATE VALVE
GYP. BD	GYPSUM BOARD
H.P.	HIGH POINT
H.R.	HANDRAIL
HB	HOSE BIBB
HCB	HOLLOW CONCRETE BOX
HD	HEAD
HDPE	HIGH DENSITY POLYETHYLENE
HEX	HEXAGON
HOR	HORIZONTAL
HR	HOUR
HT	HEIGHT
HVAC	HEATING, VENTILATION & AIR CONDITIONING
HW	HOT WATER
I.D.	INSIDE FACE
IN	INCHES
INC	INCLUDING
INV	INVERT
JAN	JANITOR
JT	JOINT
L OR LGTH	LENGTH
L.P.	LOW POINT
LAV	LAVATORY
LBS	POUNDS
LF	LINEAR FEET
LT	LEFT OR LIGHT
LO	LITHOCARPUS DENSIFLOROUS
MATL	MATERIAL
MAX	MAXIMUM
MBGR	METAL BEAM GUARD RAILING
MECH	MECHANICAL
MET	METAL
MFR	MANUFACTURER
MH	MANHOLE
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MT	METAL THRESHOLD
(N)	NEW
N	NORTH
N/A	NOT APPLICABLE
NEMA	NATIONAL ELECTRICAL MANUFACTURERS ASSOCIATION
NF	NEAR FACE
NIC	NOT IN CONTRACT
NO.	NUMBER
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OG	ORIGINAL GROUND OPPOSITE HAND
O-O	OUT - OUT
OPG	OPENING
OPP	OPPOSITE
OSHA	OCCUPATIONAL SAFETY & HEALTH ADMINISTRATION
PA	PLANTING AREA
P/L	PROPERTY LINE
PC	POINT OF CURVE
PCC	PORTLAND CEMENT CONC.
PI	POINT OF INTERSECTION
PWD	PLYWOOD
POT	POINT OF TANGENCY
PNL	PANEL
PRV	PRESSURE REDUCING VALVE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT
PTD	PAPER TOWEL DISPENSER
PTDF	PRESSURE-TREATED DOUGLAS FIR
PWD	PLYWOOD
PVC	POLYVINYL CHLORIDE
R.	RADIUS
R/W	RIGHT OF WAY
RCP	REINFORCED CONCRETE PIPE
RCV	REMOTE CONTROL VALVE
RI	RADIUS OR RISER
RD	ROAD
REF	REFERENCE
REINF	REINFORCEMENT
REP	REPRESENTATIVE
REQ'D	REQUIRED
RLD	ROCK LINED DITCH
ROT	ROUTE OF TRAVEL
RM	ROOM
RT	RIGHT
RV	RECREATION VEHICLE
RWD	REDWOOD
SCD	SEAT COVER DISPENSER
S	SEWER
S. STL OR S.S.	STAINLESS STEEL
S=	SLOPE
S4S	SURFACED 4 SIDES
SBR	SEQUENCING BATCH REACTOR
SCD	SEAT COVER DISPENSER
SCH	SCHEDULE
SD	SOAP DISPENSER
SDI	SUBSURFACE DRIP IRRIGATION
SECT.	SECTION
SF	SQUARE FEET
SHR	SHOWER
SHT	SHEET
SIM.	SIMILAR
SND	SANITARY NAPKIN DISPOSAL
SP	SPACES
SPEC'S	SPECIFICATIONS
SQ	SQUARE
SS	SANITARY SEWER STAINLESS STEEL
SSMH	SANITARY SEWER MANHOLE
STA	STATION
STAG	STAGGER
STD	STANDARD
STIFF.	STIFFENER
STL	STEEL
STRUCT	STRUCTURE
SV	SOLENOID VALVE
SYM	SYMMETRICAL
T	TANGENT
T&B	TOP & BOTTOM
T&G	TONGUE AND GROOVE
TDH	TOTAL DYNAMIC HEAD
T.O.	TOP OF
TOC	TOP OF CURB
T.O.M.W.	TOP OF MASONRY WALL
TOW	TOP OF WALL
TW	TOP OF WALL
TOS	TOP OF STEP
TPH	TOILET PAPER HOLDER
TRD	TREAD
TPZ	TREE PROTECTION ZONE
TYP, (TYP.)	TYPICAL
UG	UNDERGROUND
UNO	UNLESS NOTED OTHERWISE
VC	VERTICAL CURVE
VCP	VITRIFIED CLAY PIPE
VERT	VERTICAL
VPI	VERTICAL POINT OF INTERSECTION
VTR	VENT THROUGH ROOF
W	WATER
W/	WITH
W/H	WATER HEATER
W/O	WITHOUT
WD	WOOD
WL	WATER LINE
WP	WORKING POINT
WR	WASTE RECEPTICLE
WS	WATER SURFACE OR WATER STOP
WSA	WATER SERVICE ASSEMBLY
WTP	WATER TREATMENT PLANT
WTR	WATER
WV	WATER VALVE
WWF	WELDED WIRE FABRIC
WWTP	WASTEWATER TREATMENT PLANT

OFFICE OF THE STATE FIRE MARSHAL
APPROVED FOR PARKS ONLY
Reviewed by: _____
David Nye, OSFM
JAN 09 2018
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CALIFORNIA STATE PARKS
SINCE 1864
ACQUISITION & DEVELOPMENT DIVISION
One Capitol Mall
Sacramento, CA
95814-3229

LICENSED ARCHITECT
GIL DE LA PENA
No. C21464
Ren 1.31.19
STATE OF CALIFORNIA
Jan.2018

CALIFORNIA STATE FIRE MARSHAL- APPROVED

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Reviewed by _____ Date _____

DPR ACCESS COMPLIANCE REVIEW
ACCESSIBILITY SECTION
CERTIFICATION # 18-025
Reviewed by _____ Date 1/4/18

ACCESSIBILITY COMPLIANCE AND STATE FIRE MARSHAL SIGNED ORIGINALS ARE ON FILE AT THE DEPARTMENT OF PARKS AND RECREATION, NORTHERN SERVICE CENTER

DESIGNED:	DESIGNER
DRAWN:	STAFF
CHECKED:	SUPERVISOR
DATE:	DECEMBER 2017

REVISIONS	
	DATE

SOUTH YUBA RIVER STATE PARK
HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

ABBREVIATIONS

DRAWING NO.
30419.002

SHEET NO.
G2.0
002 OF 030

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CD'S

GENERAL NOTES

APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE

GENERAL NOTES
100SN001-1

1. INTERPRETATION OF DRAWINGS & SPECIFICATIONS
   - A. FOR CONVENIENCE, SPECIFICATIONS HAVE BEEN PREPARED FOR THIS PROJECT AND ARE ARRANGED IN SEVERAL SECTIONS, BUT SUCH SEPARATION SHALL NOT BE CONSIDERED AS THE LIMITS OF THE WORK REQUIRED OF ANY SEPARATE TRADE. THE TERMS AND CONDITIONS OF SUCH LIMITATIONS ARE WHOLLY BETWEEN THE CONTRACTOR AND HIS SUBCONTRACTORS.
   - B. IN GENERAL, THE WORKING DETAILS WILL INDICATE DIMENSIONS, POSITION AND KIND OF CONSTRUCTION, AND THE SPECIFICATIONS, QUALITIES AND METHODS. ANY WORK INDICATED ON THE WORKING DETAILS AND NOT MENTIONED IN THE SPECIFICATIONS, OR VICE VERSA, SHALL BE FURNISHED AS THOUGH FULLY SET FORTH IN BOTH. WORK NOT PARTICULARLY DETAILED, MARKED OR SPECIFIED, SHALL BE THE SAME AS SIMILAR PARTS THAT ARE DETAILED, MARKED OR SPECIFIED. IF CONFLICTS OCCUR ON DRAWINGS AND/OR SPECIFICATIONS, THE MOST EXPENSIVE MATERIALS OR METHODS WILL PREVAIL.
   - C. SHOULD AN ERROR APPEAR IN THE WORKING DETAILS OR SPECIFICATIONS OR IN WORK DONE BY OTHERS AFFECTING THIS WORK, THE CONTRACTOR SHALL NOTIFY THE STATES' REPRESENTATIVE AT ONCE AND IN WRITING. IF THE CONTRACTOR PROCEEDS WITH THE WORK SO AFFECTED WITHOUT HAVING GIVEN SUCH WRITTEN NOTICE AND WITHOUT RECEIVING THE NECESSARY APPROVAL, DECISION OR INSTRUCTIONS IN WRITING FROM THE STATE, THEN HE SHALL HAVE NO VALID CLAIM AGAINST THE STATE, FOR THE COST OF SO PROCEEDING AND SHALL MAKE GOOD ANY RESULTING DAMAGE OR DEFECT. NO VERBAL APPROVAL, DECISION, OR INSTRUCTION SHALL BE VALID OR BE THE BASIS FOR ANY CLAIM AGAINST THE STATE, ITS OFFICERS, EMPLOYEES OR AGENTS. THE FOREGOING INCLUDES TYPICAL ERRORS IN THE SPECIFICATIONS OR NOTATIONAL ERRORS IN THE WORKING DETAILS WHERE THE INTERPRETATION IS DOUBTFUL OR WHERE THE ERROR IS SUFFICIENTLY APPARENT AS TO PLACE A REASONABLY PRUDENT CONTRACTOR ON NOTICE THAT, SHOULD HE ELECT TO PROCEED, HE IS DOING SO AT HIS OWN RISK.
2. CONSTRUCTION SHALL CONFORM TO ALL APPLICABLE CODES AND REGULATIONS.
3. SHOP DRAWING NOTE:
   - A. WHEN NOT ADRESSED BY DIVISION 1 OF THE SPECIFICATIONS, PAPER FORMAT STRUCTURAL SHOP DRAWINGS SHALL BE SUBMITTED IN THE FORM OF THREE COPIES MINIMUM OF EACH SHEET.
   - B. THE PURPOSE OF SHOP DRAWINGS SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE STRUCTURAL ENGINEER THAT HE UNDERSTANDS THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL HE INTENDS TO FURNISH AND INSTALL, AND BY DETAILING THE FABRICATION AND INSTALLATION METHODS HE INTENDS TO USE ON A STAND ALONE SET OF DOCUMENTS. DUPLICATION OF DESIGN DOCUMENTS FOR THE PURPOSE OF SHOP DRAWINGS IS NOT ACCEPTABLE.
   - C. PRIOR TO FABRICATION, SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW BY THE STRUCTURAL ENGINEER. SHOP DRAWING SUBMITTALS SHALL INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO, STRUCTURAL STEEL, REINFORCING STEEL, & GLUED LAMINATED BEAMS.
   - D. PRIOR TO SUBMISSION THE CONTRACTOR SHALL REVIEW ALL SUBMITTALS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND SHALL STAMP SUBMITTALS AS BEING "REVIEWED FOR CONFORMANCE".
   - E. SHOP DRAWINGS SUBMITTALS PROCESSED BY THE STRUCTURAL ENGINEER ARE NOT CHANGE ORDERS.
   - F. ANY DETAIL ON THE SHOP DRAWING THAT DEVIATES FROM THE CONTRACT DOCUMENTS SHALL CLEARLY BE MARKED WITH THE NOTE "THIS A CHANGE".
   - G. SHOP DRAWINGS OR CALCULATIONS SUBMITTED FOR REVIEW THAT REQUIRE RESUBMITTAL FOR RE-REVIEW SHALL BE BILLED HOURLY FOR SUCH TIME TO THE GENERAL CONTRACTOR. RE-REVIEW WILL NOT PROCEED WITHOUT WRITTEN APPROVAL FROM THE GENERAL CONTRACTOR FOR ADDITIONAL ENGINEERING REVIEW SERVICES.
4. SAFETY NOTE:
   - A. IT IS THE CONTRACTORS RESPONSIBILITY TO COMPLY WITH THE PERTINENT SECTIONS, AS THEY APPLY TO THIS PROJECT, OF THE "CONSTRUCTION SAFETY ORDERS" ISSUED BY THE STATE OF CALIFORNIA LATEST EDITION, AND ALL OSHA REQUIREMENTS.
   - B. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE DESIGN AND CONSTRUCTION OF ALL FORMS AND SHORING REQUIRED. SHORING INDICATIONS (LOCATION, DIRECTION, DURATION, ETC.) ARE ONLY SHOWN ON THE STRUCTURAL DRWGS WHEN REQUIRED TO IMPLEMENT THE DESIGN INTENT OF THE FINAL WORK PRODUCT. DETERMINATION WHETHER SHORING IS REQUIRED FOR TEMPORARY OR INTERMEDIATE CONDITIONS DURING COONSTRUCTION IS WHOLLY THE RESPOSIBILITY OF THE CONTRACTOR.
   - C. THE STATE AND THE STRUCTURAL ENGINEER DO NOT ACCEPT ANY RESPONSIBILITY FOR THE CONTRACTOR'S FAILURE TO COMPLY WITH THESE REQUIREMENTS.
5. THE CONTRACTOR SHALL NOTIFY THE STATE'S REPRESENTATIVE AND STRUCTURAL ENGINEER WHERE A CONFLICT OR DISCREPANCY OCCURS BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER PORTION OF THE CONTRACT DOCUMENTS OR EXISTING FIELD CONDITIONS. SUCH NOTIFICATION SHALL BE GIVEN IN DUE TIME SO AS NOT TO AFFECT THE CONSTRUCTION SCHEDULE. IN CASE OF A CONFLICT BETWEEN STRUCTURAL DRAWINGS AND SPECIFICATIONS THE MORE RESTRICTIVE CONDITION SHALL TAKE PRECEDENCE UNLESS WRITTEN APPROVAL HAS BEEN GIVEN FOR THE LEAST RESTRICTIVE. CONTRACTOR SHALL VERIFY ALL DIMENSIONS WITH STATES REPRESENTATIVE PRIOR TO COMMENCING ANY WORK.
6. WHERE NO SPECIFIC DETAIL IS SHOWN, THE CONSTRUCTION SHALL BE IDENTICAL OR SIMILAR TO THAT INDICATED FOR LIKE CASES OF CONSTRUCTION ON THIS PROJECT. SHOULD THERE BE ANY QUESTION, CONTACT THE STATES REPRESENTATIVE AND STRUCTURAL ENGINEER PRIOR TO PROCEEDING.
7. WHEN CONSTRUCTION ATTACHES TO OR IS WITHIN AN EXISTING BUILDING, A COMPLETE SET OF DRAWINGS OF THE EXISTING BUILDING SHALL BE KEPT ON THE JOB SITE. CONTRACTOR TO OBTAIN THESE DRAWINGS FROM THE OWNER (IF THEY ARE AVAILABLE).
8. ANY SUBSTITUTIONS OR ALTERNATE METHODS FOR STRUCTURAL MEMBERS, HARDWARE OR DETAILS SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER. SUCH REVIEW WILL BE BILLED ON A TIME AND MATERIALS BASIS TO THE GENERAL CONTRACTOR WITH NO GUARANTEE THAT THE SUBSTITUTION WILL BE ALLOWED.
9. DO NOT SCALE DRAWINGS. CONTACT THE STATES REPRESENTATIVE OR STRUCTURAL ENGINEER FOR ANY DIMENSIONS NOT SHOWN.
10. THESE DRAWINGS ARE NOT COMPLETE UNTIL REVIEWED AND ACCEPTED BY STATE OFFICIALS AND SIGNED BY THE STATES' REPRESENTATIVE AND THE STRUCTURAL ENGINEER.

ABBREVIATIONS
100SN003-1

AB	----------	ANCHOR BOLT	LS	----------	LAG SCREW
ABV	----------	ABOVE	MB	----------	MACHINE BOLT
AR	----------	ANCHOR ROD	MFR	----------	MANUFACTURER
BLW	----------	BELOW	MI	----------	MALLEABLE IRON
BOF	----------	BOTTOM OF FOOTING	MTL	----------	METAL
BRG	----------	BEARING	(N)	----------	NEW
BET	----------	BETWEEN	NIC	----------	NOT IN CONTRACT
C/C	----------	CENTER TO CENTER	NS	----------	NEAR SIDE
CTJ	----------	CONTROL JOINT	NTS	----------	NOT TO SCALE
CJP	----------	COMPLETE JOINT PENETRATION	NW	----------	NORMAL WEIGHT
CLR	----------	CLEAR	OH	----------	OPPOSITE HAND
CMU	----------	CONCRETE MASONRY UNIT	PC	----------	PIECE
CJ	----------	CONSTRUCTION JOINT	PJP	----------	PARTIAL JOINT PENETRATION
CONT	----------	CONTINUOUS	SC	----------	SLIP CRITICAL
CONTR	----------	CONTRACTOR	SHTG	----------	SHEATHING
CSK	----------	COUNTERSINK	SIM	----------	SIMILAR
DL	----------	DEAD LOAD	SMS	----------	SHEET METAL SCREW
DO	----------	DITTO	SP	----------	STRUCTURAL PANEL
DWG	----------	DRAWING	STIFF	----------	STIFFENER
(E)	----------	EXISTING	STGRD	----------	STAGGERED
EF	----------	EACH FACE	STL	----------	STEEL
EJ	----------	EXPANSION JOINT	T&B	----------	TOP & BOTTOM
EL	----------	ELEVATION	THD	----------	THREADED
EN	----------	EDGE NAILING	TO	----------	TOP OF
EOS	----------	EDGE OF SLAB	TOC	----------	TOP OF CONCRETE (SLAB UNO)
EQ	----------	EQUAL	TOF	----------	TOP OF FOOTING OR TOP OF FRAMING
EW	----------	EACH WAY	TOS	----------	TOP OF STEEL
EWEF	----------	EACH WAY EACH FACE	TOW	----------	TOP OF WALL
FB	----------	FACE OF BLOCK(OR BRICK) OR FLAT BAR	UNO	----------	UNLESS NOTED OTHERWISE
FC	----------	FACE OF CONCRETE	W/	----------	WITH
FF	----------	FINISH FLOOR	W/O	----------	WITHOUT
FS	----------	FACE OF STUD OR FAR SIDE	WP	----------	WORK POINT
GA	----------	GAUGE OR GAGE	WS	----------	WOOD SCREW
HB	----------	HEADED BOLT	WWF	----------	WELDED WIRE FABRIC
HDR	----------	HEADER	CL	----------	CENTERLINE
HSB	----------	HIGH STRENGTH BOLT	PL	----------	PLATE
HSS	----------	HOLLOW STRUCTURAL SECTION	WF	----------	WIDE FLANGE
HT	----------	HEIGHT	#	----------	NUMBER OR POUNDS
JH	----------	JOIST HANGER	SQ	----------	SQUARE
LL	----------	LIVE LOAD	Ø	----------	ROUND OR DIAMETER
LLH	----------	LONG LEG HORIZONTAL	(A)	----------	"MEMBER" ABOVE
LLV	----------	LONG LEG VERTICAL	(B)	----------	"MEMBER" BELOW

DESIGN CRITERIA
100SN002-1

1. CODES AND STANDARDS
   2013 CALIFORNIA BUILDING CODE
   ASCE 7-10
   ACI 318-11
   AISC 360-10, AISC 341-10
   2012 NDS, 2008 SDPWS

2. VERTICAL LOADS
   BRIDGE DECK LIVE LOAD = 65 PSF
3. SOILS VALUES
   FOOTING
   MINIMUM DEPTH = 18"
   MINIMUM WIDTH = 24"
   PASSIVE RESISTANCE = 150 PCF
   FRICTION = 0.40
   (REDUCE FRICTION BY 50% WHERE COMBINED WITH PASSIVE)
   MICROPILES (SEE 1 / S0.03)
4. LATERAL LOADS
   SEISMIC:
   SITE CLASS: C
   Ss = 0.580 ; S_{DS} = 0.452
   S_1= 0.247; S_{D1} = 0.256
   R = 1.25
   Ωo = 2.0 ; Cd = 2.5
   I_E = 1.0 TYPICAL
   RISK CATEGORY: II
   SEISMIC DESIGN CATEGORY: D
   SEISMIC BASE SHEAR
   = 96 KIPS (N/S DIR.)
   = 96 KIPS (E/W DIR.)
   SEISMIC FORCE RESISTING SYSTEM:
   CONCRETE ABUTMENT (ALL OTHER STRUCTURES)
   LATERAL ANALYSIS PROCEDURE:
   LINEAR STATIC
   WIND:
   V_{ULT} =110 MPH , V_{ASD} = 85 MPH
   RISK CATEGORY: II
   EXPOSURE CATEGORY: C

STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS AND TESTING
100SN004-1

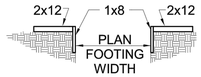
1. SPECIAL INSPECTIONS AND TESTING SHALL BE PROVIDED BY AN INSPECTION AGENCY, EMPLOYED BY THE CONTRACTOR, AND QUALIFIED BY THE BUILDING OFFICIAL TO INSPECT THE PARTICULAR TYPE OF CONSTRUCTION. TESTS AND INSPECTIONS, AS REQUIRED BY SECTIONS 110, 1704, 1705.10, 1705.11 AND 1705.12 OF THE 2013 CBC, SHALL BE PERFORMED DURING CONSTRUCTION ON THE TYPES OF WORK LISTED BELOW:

	INSPECTIONS/TESTING
■ STRUCTURAL STEEL CONSTRUCTION	SECTION 1705.2.1
■ CONCRETE CONSTRUCTION	SECTION 1705.3
□ MASONRY CONSTRUCTION-LEVEL B	SECTION 1705.4
■ SOILS	SECTION 1705.6
■ POST-INSTALLED ANCHORS	SEE DRILLED-IN ANCHOR NOTES
■ BRIDGE ROD TUNING	SEE SPECIFICATIONS
■ MICROPILES	SEE 4/S5.03

2. INSPECTIONS SHALL BE CONTINUOUS OR PERIODIC AS NOTED FOR THE INDIVIDUAL MATERIAL OR COMPONENT INSPECTION SECTIONS AND TABLES NOTED ABOVE.
3. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS TO THE STATE'S REPRESENTATIVE AND THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE REPORTS SHALL INDICATE WHETHER WORK INSPECTED CONFORMED TO THE CONSTRUCTION DOCUMENTS. ANY DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE STATE'S REPRESENTATIVE AND THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.
4. ALL SPECIAL INSPECTION AGENCIES / INDIVIDUALS AND SHOP FABRICATORS SHALL BE APPROVED BY THE STATE'S REPRESENTATIVE PRIOR TO COMMENCEMENT OF WORK.
5. TESTING AND INSPECTION RECORDS SHALL BE RETAINED UNTIL COMPLETION OF CONSTRUCTION.
6. ALL SOILS AND FOUNDATION EXCAVATION INSPECTIONS SHALL BE BY THE GEOTECHNICAL ENGINEER OF RECORD.
7. FOR TESTING AND INSPECTION REQUIREMENTS FOR NON-STRUCTURAL MATERIALS AND COMPONENTS, SEE CONSTRUCTION DOCUMENTS AND COMPLY WITH CHAPTER 17 OF THE 2013 CBC.

FOUNDATIONS
200SN001-1

1. ALL FOUNDATION WORK SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE SOILS REPORT #59030-05-41 BY GEOCON CONSULTANTS, INC DATED MAY 2015.
2. FOUNDATIONS SHALL BE MICROPILES. SEE 4/S5.03
3. ALL FILLING, BACKFILLING AND COMPACTION SHALL BE DONE UNDER THE OBSERVATION OF A REPRESENTATIVE OF THE SOILS ENGINEER OF RECORD AND MUST BE COMPACTED TO THE MINIMUM DENSITY SPECIFIED IN ACCORDANCE WITH THE PROCEDURE OUTLINED IN THE SOILS REPORT.
4. BUILDING PAD CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE SOILS REPORT. THE EXTENT AND DEPTH OF OVEREXCAVATION AND PLACEMENT OF ENGINEERED FILL SHALL AT A MINIMUM BE AS SHOWN ON THE PLANS. FINAL DEPTH AND EXTENT OF EXCAVATION AND FILL SHALL BE DETERMINED AT TIME OF CONSTRUCTION BY A REPRESENTATIVE OF THE SOILS ENGINEER. FOUNDATION DEPTHS INDICATED ON PLANS ARE FOR ESTIMATING PURPOSES ONLY.
5. BOTTOMS OF ALL FOUNDATIONS SHALL BE LEVEL. CHANGES IN BOTTOM OF FOUNDATION ELEVATION SHALL BE MADE ACCORDING TO STEPPED FOOTING DETAIL ON THE TYPICAL DETAIL SHEET. FOUNDATION CONCRETE MAY BE PLACED DIRECTLY INTO NEAT EXCAVATIONS PROVIDED
6. THE EXCAVATIONS ARE STABLE (AS DETERMINED BY A REPRESENTATIVE OF THE SOILS ENGINEER). OTHERWISE, FOUNDATIONS SHALL BE FULLY FORMED. USE MINIMUM PLANKING SHOWN TO PROTECT AGAINST SLOUGHING, AS REQUIRED. PLANKING DOES NOT REPLACE FORMWORK REQUIRED TO STABILIZE EXCAVATION. - SEE BELOW.
7. THE SURFACE OF ALL HORIZONTAL CONSTRUCTION JOINTS SHALL BE CLEANED & ROUGHENED BY EXPOSING CLEAN AGGREGATE SOLIDLY EMBEDDED IN MORTAR MATRIX.
8. NOTIFY THE STRUCTURAL ENGINEER 48 HOURS BEFORE CASTING FOUNDATIONS.
9. A REPRESENTATIVE OF THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT:
   A. THE FOUNDATION EXCAVATION DEPTH AND MATERIAL ARE ADEQUATE TO ACHIEVE DESIGN BEARING CAPACITY; AND FORMING COMPLY WITH THE SOILS REPORT AND APPROVED PLAN.



WOOD
600SN001-1

. ALL STRUCTURAL WOOD SHALL CONFORM WITH THE FOLLOWING SPECIFICATIONS:
   DOUGLAS FIR - COAST REGION - WCLIB GRADING RULES #17 OR WWPA STANDARD GRADING RULES FOR WESTERN LUMBER.
   2X, 3X AND 4X MEMBERS - SELECT STRUCTURAL UNO.
   6X AND LARGER MEMBERS - SELECT STRUCTURAL UNO.
   MEMBERS 3X AND LARGER SHALL BE FREE OF HEART CENTER
   REDWOOD - CALIFORNIA REDWOOD, RIS.
   PRESSURE TREATED DOUGLAS FIR - TYPE AS APPROPRIATE FOR EXTERIOR ABOVE GROUND USE AS SPECIFIED, BY AWPA.
2. ALL DOUGLAS FIR WOOD IN DIRECT CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED, EXCEPT LEDGERS, WHICH DO NOT NEED TO BE PRESSURE TREATED.
3. FIELD CUTS AND BOLT HOLES IN PRESSURE TREATED WOOD SHALL BE PROTECTED IN ACCORDANCE WITH AWPA STANDARD M4.
4. THE MOISTURE CONTENT OF 2X MATERIAL AT TIME OF INITIAL USE SHALL BE LESS THAN 12%. THE MOISTURE CONTENT OF LUMBER 3X AND LARGER AT TIME OF INITIAL USE SHALL BE LESS THAN 19% UNO.
5. HOLES FOR BOLTS IN WOOD SHALL BE BORED WITH A BIT OF THE SAME NOMINAL DIAMETER AS THE BOLT PLUS 1/16".
6. HOLES FOR LAG SCREWS SHALL BE FIRST BORED TO THE SAME DIAMETER AND DEPTH AS THE UNTHREADED SHANK. THE THREADED PORTION SHALL BE DRILLED TO 50% OF THE SHANK DIAMETER UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS.
7. LAG SCREWS AND WOOD SCREWS SHALL BE SCREWED AND NOT DRIVEN INTO PLACE. SOAP MAY BE USED TO LUBRICATE SCREWS.
8. ALL BOLTS AND LAG SCREWS SHALL BE PROVIDED WITH METAL WASHERS UNDER HEADS AND NUTS WHICH BEAR ON WOOD. APPLIES ALSO TO EXPANDING AND ADHESIVE FASTENERS. SQUARE STEEL WASHERS SHALL BE SIMPSON BP OR BPS TYPE (3" SQ. MIN. W/ STD CUT WASHER WHERE REQ'D BY CODE). MALLEABLE IRON (MI) WASHERS SHALL BE SQUARE AND CAST. STANDARD FLAT WASHERS SHALL BE PER ANSI B18.22.1 TYPE A - WIDE PATTERN. ALL WASHERS FOR SILL ANCHORS SHALL BE SQUARE STEEL AND SHALL BE HOT-DIPPED GALVANIZED.
9. ALL BOLTS AND LAG SCREWS SHALL BE TIGHTENED ON INSTALLATION AND RETIGHTENED BEFORE CLOSING IN OR AT COMPLETION OF JOB.
10. ALL BOLTS SHALL BE FULL NOMINAL DIMENSION AT UNTHREADED PORTION. NO UPSET THREADED BOLTS ALLOWED.
11. ALL FASTENERS SHALL BE HOT-DIPPED GALVANIZED.
12. FRAMING HARDWARE SHOWN ON THE PLANS IS SIMPSON STRONG-TIE. USE FRAMING HARDWARE AS MANUFACTURED BY SIMPSON COMPANY OR EQUIVALENT. PRIOR TO INSTALLATION OF ANY NON-SIMPSON HARDWARE, THE CONTRACTOR SHALL SUBMIT A LIST OF ALL DETAIL REFERENCES WHERE A HARDWARE SUBSTITUTION IS PROPOSED, THE DESIGNATION FOR THE SIMPSON ITEM AND NON-SIMPSON PROPOSED EQUIVALENT AND AN ICC REPORT FOR EACH SUBSTITUTION ITEM.

STRUCTURAL SHEET INDEX	
S0.01	GENERAL NOTES
S0.02	GENERAL NOTES
S0.03	GENERAL NOTES
S0.04	TYPICAL DETAILS
S1.01	SHORING PLAN
S1.02	DEMOLITION PLAN
S2.01	BRIDGE FOUNDATION PLAN
S2.02	SOUTH BRIDGE ABUTMENT PLAN
S2.03	NORTH BRIDGE ABUTMENT PLAN
S2.04	BRIDGE FLOOR DECK & TOP CHORD BRACING PLAN
S2.05	ROOF FRAMING PLAN
S3.01	SECTIONS
S3.02	SECTIONS
S3.03	SECTIONS
S4.01	TRUSS ELEVATIONS
S4.02	TRUSS REPAIR ELEVATIONS
S5.01	DETAILS
S5.02	DETAILS
S5.03	DETAILS
S5.04	DETAILS

OFFICE OF THE STATE FIRE MARSHAL
APPROVED FIRE AND PANIC ONLY
Reviewed by: David Nye, OSFM
JAN 09 2018
Approval of this plan does not authorize or approve any omission or deviation from applicable regulations. Final approval is subject to field inspection. One set of approved plans shall be available on the project site at all times.

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LAWRENCE E. JONES
No. 3139
STRUCTURAL
STATE OF CALIFORNIA

CALIFORNIA STATE FIRE MARSHAL- APPROVED

Approval of this plan does not authorize or approve any omission of deviation from applicable regulations. Final approval is subject to field inspection. One set of approved plans shall be available on the project site at all times.

Reviewed by _____ Date _____

DPR ACCESS COMPLIANCE REVIEW
ACCESSIBILITY SECTION
CERTIFICATION # 18-005
Reviewed by _____ Date 1/16/18

ACCESSIBILITY COMPLIANCE AND STATE FIRE MARSHAL SIGNED ORIGINALS ARE ON FILE AT THE DEPARTMENT OF PARKS AND RECREATION, NORTHERN SERVICE CENTER

DESIGNED:
DRAWN:
CHECKED:
DATE: Issue Date

REVISIONS
DATE

SOUTH YUBA RIVER STATE PARK
HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

GENERAL NOTES

DRAWING NO.
30419.003

SHEET NO.
S0.01

003 OF 030

File: Q:\South_Yuba_River_SP\202193-Covered_Bridge_Stabilization\10_Architectural_Working_Files\ArchDrawingFiles\Architectural\S0.01 GENERAL NOTES.dwg Layout: GENERAL NOTES Date: February 02 2018 - 3:13 pm User: GDELA

CD'S

GENERAL NOTES

APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE

CONCRETE

3000SN001-1

1. STRUCTURAL CONCRETE SHALL ATTAIN 28 DAY COMPRESSIVE STRENGTH AS REQUIRED IN NOTE #28. MAXIMUM SLUMP SHALL NOT EXCEED 4 INCHES.
2. CONCRETE MIX DESIGNS SHALL BE PREPARED BY A REGISTERED CIVIL ENGINEER, REVIEWED BY CONTRACTOR'S TESTING LABORATORY AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW.
3. CEMENTITIOUS MATERIALS:
   CEMENT SHALL CONFORM TO ASTM C-150 TYPE I OR II.
   FLY ASH SHALL CONFORM TO ASTM C-618. MAX. QUANTITY OF FLY ASH SHALL BE AS GIVEN IN SPECS (15% MAX UNO).
4. CONCRETE AGGREGATES SHALL CONFORM TO ASTM C-33 FOR NORMAL WEIGHT CONCRETE AND ASTM C-330 FOR LIGHTWEIGHT CONCRETE.
5. NON-SHRINK GROUT OR DRYPACK SHALL CONSIST OF A PREMIXED NONMETALLIC FORMULA.
6. REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60 FOR #3 AND LARGER, EXCEPT REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A-706. CONTRACTOR SHALL SUBMIT REBAR MILL CERTIFICATES. ALL REINFORCING SHALL BE GREEN EPOXY COATED AND CONFORM TO ASTM A-775. EPOXY COATED BARS SHALL BE INSPECTED PRIOR TO CONCRETE PLACEMENT. ANY DAMAGED COATINGS SHOULD BE REPAIRED ACCORDING TO ASTM A-775. PLASTIC HEADED VIBRATORS SHOULD BE USED FOR CONCRETE CONSOLIDATION. FIELD TOUCH-UP ALL END CUTS AND DAMAGE.
7. ALL PREHEATING AND WELDING OF REINFORCING BARS SHALL BE DONE IN ACCORDANCE WITH AWS D1.4 LATEST EDITION AND SHALL BE CONTINUOUSLY INSPECTED BY A QUALIFIED LABORATORY. CONTRACTOR SHALL FURNISH WPS FOR ALL REBAR WELDING TO THE LABORATORY.
8. REINFORCING STEEL SHALL BE FABRICATED ACCORDING TO "MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION".
9. WIRE FABRIC SHALL CONFORM TO ASTM A-185.
10. DIMENSIONS SHOWN FOR LOCATION OF REINFORCING ARE TO THE FACE OF BARS LISTED AND DENOTE CLEAR COVERAGE. NON-PRESTRESSED, CAST-IN-PLACE CONCRETE COVERAGE SHALL BE AS FOLLOWS, UNO:
    CONCRETE DEPOSITED DIRECTLY AGAINST GROUND (EXCEPT SLABS)--- 3"
    CONCRETE EXPOSED TO GROUND OR WEATHER BUT PLACED IN FORMS:
        #5 AND SMALLER---------- 1-1/2"
        #6 AND LARGER---------- 2"
    BEAMS & COLUMNS (TIES)---------- 1-1/2"
    BEAMS & COLUMNS (MAIN REINFORCING)---------- 2"
    CAST-IN-PLACE WALLS (EXTERIOR FACE & SOIL SIDE)--------- SEE ABOVE
    CAST-IN-PLACE WALLS (INTERIOR FACE-#11 & SMALLER)------- 3/4"
    SLABS (ON FORMS)---------- 3/4"
    SLABS (ON GROUND)---------- 2" CLEAR FROM TOP UNO
11. SPLICES IN CONTINUOUS REINFORCEMENT SHALL BE LAPPED UNO, SEE SCHEDULE THIS SHEET. SPLICES IN ADJACENT BARS SHALL BE GREATER THAN 5'-0" APART. SPLICE CONTINUOUS BARS IN SOIL-BEARING GRADE BEAMS, STRUCTURAL SLABS ON GRADE AND MAT FOUNDATIONS AS FOLLOWS UNO: TOP BARS AT CENTERLINE OF SUPPORT; BOTTOM BARS AT MID-SPAN. SPLICE CONTINUOUS BARS IN ELEVATED SLABS AND BEAMS, ETC. AS FOLLOWS UNO: TOP BARS AT MID-SPAN; BOTTOM BARS AT CENTERLINE OF SUPPORT. ALL BARS SIZE #14 AND LARGER SHALL BE CONTINUOUS FOR FULL LENGTH SHOWN OR SPLICED WITH MECHANICAL COUPLERS AS NOTED IN DETAILS. SPLICES IN WWF SHALL BE 1-1/2 MESHES WIDE.
12. THE MINIMUM CLEAR SPACING BETWEEN PARALLEL BARS IN A LAYER SHALL NOT BE LESS THAN THE LARGER OF BAR DIAMETER, 1", OR 33% GREATER THAN THE MAXIMUM AGGREGATE SIZE (NOMINAL), WHICHEVER IS GREATEST. THIS REQUIREMENT ALSO APPLIES TO THE CLEAR SPACING BETWEEN DIFFERENT LAYERS OF PARALLEL BARS AND TO THE CLEAR DISTANCE BETWEEN A CONTACT LAP SPLICE AND ADJACENT SPLICES OR BARS.
13. ALL HOOKS SHALL BE STANDARD HOOKS UNLESS OTHERWISE SHOWN OR NOTED. AT WALLS, PROVIDE HOOKS AT ENDS OF ALL REINFORCING AT ENDS, CORNERS AND INTERSECTIONS, UNO.
14. CONSTRUCTION JOINTS SHALL BE MADE ROUGH AND ALL LAITANCE REMOVED FROM THE SURFACE. CONCRETE MAY BE ROUGHENED BY CHIPPING THE ENTIRE SURFACE, SAND BLASTING, OR RAKING THE SURFACE TO PROVIDE 1/4" DEEP DEFORMATIONS.
15. REMOVE ALL DEBRIS FROM FORMS BEFORE CASTING ANY CONCRETE.
16. REINFORCING, DOWELS, BOLTS, ANCHORS, SLEEVES, ETC. TO BE EMBEDDED IN CONCRETE SHALL BE SECURELY POSITIONED BEFORE PLACING CONCRETE.
17. ANCHOR BOLTS (AB'S) CAST IN CONCRETE OR MASONRY FOR WALL SILL AND LEDGER\APPLICATIONS SHALL BE HEADED BOLTS WITH CUT THREADS CONFORMING TO ASTM A307, UNO. REFER TO "WOOD" NOTES FOR ADDITIONAL REQUIREMENTS FOR BOLTS IN CONTACT WITH PRESSURE TREATED OR FIRE RETARDANT MATERIAL. REFER TO 'STRUCTURAL STEEL' NOTE FOR REQUIREMENTS FOR ANCHOR RODS (AR'S) CAST IN CONCRETE FOR COLUMN BASE PLATE AND STEEL EMBED APPLICATIONS.
    WALLS SHALL BE CAST IN HORIZONTAL LAYERS OF 2'-0" MAXIMUM DEPTH.
    CONCRETE IN WALLS, PIERS OR COLUMNS SHALL SET AT LEAST 2 HOURS BEFORE PLACING CONCRETE IN BEAMS, SPANDRELS, OR SLABS SUPPORTED THEREON.
    HORIZONTAL WALL BARS IN MULTI-CURTAIN CAST IN PLACE WALLS SHALL BE STAGGERED.
    DOWEL ALL VERTICAL REINFORCING IN WALLS AND COLUMNS FROM FOUNDATION WITH SAME SIZE BAR.
    CONSOLIDATE CONCRETE PLACED IN FORMS BY MECHANICAL VIBRATING EQUIPMENT SUPPLEMENTED BY HAND-SPADING, RODDING OR TAMPING. USE EQUIPMENT AND PROCEDURES FOR CONSOLIDATION OF CONCRETE IN ACCORDANCE WITH THE RECOMMENDED PRACTICES OF ACI 309 TO SUIT THE TYPE OF CONCRETE AND PROJECT CONDITIONS. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS) SO AS TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES HOPPERS AND CHUTES OR TRUNKS OF VARIABLE LENGTHS SHALL BE USED SO THAT THE FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED 6 FEET.
    NO WOOD SPREADERS ALLOWED. NO WOOD STAKES ALLOWED IN AREAS TO BE CONCRETED.
    ADDITIONAL REINFORCING IN PRECAST OR TILT-UP PANELS REQUIRED FOR LIFTING STRESSES SHALL BE SUPPLIED BY CONTRACTOR.
    PROVIDE #5 X 4'-0" DIAGONAL REINFORCING AT TOP AND BOTTOM OF SLAB AT ALL RE-ENTRANT CORNERS TYPICAL. THIS APPLIES TO SLAB ON GRADE, CONCRETE OVER METAL DECK, AND ELEVATED STRUCTURAL SLAB CONDITIONS.
    ALL SAW CUTTING SHALL BE DONE AFTER INITIAL SET HAS OCCURRED TO AVOID TEARING OR DAMAGE BY THE SAW BLADE, BUT BEFORE INITIAL SHRINKAGE HAS OCCURRED.
    NOTIFY STRUCTURAL ENGINEER A MINIMUM OF 48 HOURS BEFORE PLACING ANY CONCRETE.
    CONCRETE STRENGTHS & MIX PROPERTIES:

ITEM	F'C @ 28 DAYS	MAX AGGR. SIZE	WEIGHT	MAX W/CM* RATIO
A. FOUNDATIONS & WALL	4000 PSI	1"	NW	0.45
B. SLAB ON GRADE	3500 PSI	1"	NW	0.45
SITE AND MISCELLANEOUS - SEE CIVIL OR ARCH'L DRAWINGS				

* W/CM = WATER : CEMENTITIOUS MATERIAL RATIO

STRUCTURAL STEEL

5000SN001-1

1. FABRICATION, ERECTION AND MATERIALS SHALL CONFORM WITH THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, THE AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, AND THE INTERNATIONAL BUILDING CODE, LATEST EDITIONS.
2. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM WITH ASTM A992. ALL OTHER STRUCTURAL STEEL ROLLED SHAPES (CHANNELS, ANGLES, ETC) AND PLATES SHALL CONFORM WITH ASTM A36, UNO.
3. STEEL PIPE SHALL CONFORM TO ASTM A53, TYPES E OR S, GRADE B.
4. ALL HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B.
5. ALL STRUCTURAL STEEL SHALL RECEIVE A MINIMUM OF ONE SHOP COAT OF RED PRIMER PAINT. DO NOT PAINT AREAS TO BE FIELD WELDED, FIREPROOFED, GALVANIZED, TO RECEIVE SLIP-CRITICAL HIGH STRENGTH BOLTS, OR TO BE EMBEDDED IN CONCRETE. PROVIDE ADDITIONAL PAINTING AS NOTED IN THE SPECIFICATIONS.
6. ALL STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND SHALL BE LEFT IN PLACE UNTIL OTHER MEANS ARE PROVIDED TO ADEQUATELY BRACE THE STRUCTURE. CONTRACTOR RESPONSIBLE FOR REVIEWING ALL BASE PLATE AND SUPPORT CONDITIONS DURING ERECTION AND BRACING AS REQUIRED. SEE AISC AND OSHA REQUIREMENTS.
7. PLACE NON-SHRINK GROUT UNDER ALL BASE PLATES BEFORE ADDING VERTICAL LOAD.
8. STRUCTURAL STEEL BELOW GRADE SHALL HAVE 3 INCHES MINIMUM OF CONCRETE COVER.
9. BOLTED CONNECTIONS SHALL CONSIST OF UNFINISHED BOLTS CONFORMING TO ASTM A307 UNLESS NOTED OTHERWISE. WHERE HIGH STRENGTH BOLTS ARE INDICATED, BOLTS CONFORMING TO ASTM A325 OR ASTM A490 AS NEEDED SHALL BE PROVIDED. ANCHOR RODS CAST IN CONCRETE OR MASONRY SHALL BE HEADED BOLTS WITH CUT THREAD, FULL DIAMETER BODY STYLE CONFORMING TO ASTM F1554 GR. 36, 55 (WELDABLE PER S1 SUPPLEMENTARY REQUIREMENTS), OR 105 AS INDICATED ON DRAWINGS. ALL BOLTED CONNECTIONS AND BASE PLATES SHALL HAVE STANDARD CUT WASHERS UNLESS NOTED OTHERWISE. WASHERS FOR BASE PLATES SHALL CONFORM TO ASTM F844 UNLESS NOTED OTHERWISE, AND SHALL BE PLACED AT TOP AND BOTTOM OF PLATE.
10. "SLIP-CRITICAL" BOLTED CONNECTIONS:
    A) "SLIP-CRITICAL" CONNECTIONS (A325SC DESIGN VALUES WITH SPECIAL INSPECTION) ARE REQUIRED AT ALL BRACED FRAME CONNECTIONS, AT ALL CONNECTIONS ALONG CHORD LINES AND DRAG LINES (AS NOTED ON PLANS), AND UNO, AT ALL BOLTS IN OVERSIZED OR SLOTTED HOLES.
    B) THE SPECIAL INSPECTOR MUST BE PRESENT DURING INSTALLATION AND TIGHTENING OPERATION OF "SLIP-CRITICAL" CONNECTIONS.
11. PROVIDE 1/2" DIAMETER STITCH BOLTS AND RING FILLS, SPACED AT NOT MORE THAN 2'-0" ON CENTER FOR ALL DOUBLE ANGLE MEMBERS.
12. AT WOOD TO STEEL PARALLEL CONTACT, BOLT WITH 1/2" DIAMETER BOLTS AT MAXIMUM 24"C/C.
13. HOLES FOR UNFINISHED BOLTS SHALL BE OF THE SAME NOMINAL DIAMETER OF THE BOLT PLUS 1/16". USE STANDARD AISC GAGE AND PITCH FOR BOLTS EXCEPT AS NOTED OTHERWISE.
14. WELDING SHALL BE DONE BY THE ELECTRIC ARC PROCESS IN ACCORDANCE WITH AMERICAN WELDING SOCIETY STANDARDS, USING ONLY CERTIFIED WELDERS. ALL GROOVE WELDS SHALL HAVE COMPLETE PENETRATION UNLESS NOTED OTHERWISE. ALL EXPOSED WELDS SHALL BE GROUND SMOOTH. ALL ELECTRODES FOR WELDING SHALL COMPLY WITH AWS CODE, E70 SERIES MINIMUM.
15. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTHS REQUIRED.
16. MINIMUM FILLET WELDS: 3/16" @ T < 1/2"
    1/4" @ T < 3/4"
    5/16" @ T > 3/4"
17. WELDING PROCEDURE SPECIFICATIONS (WPS) FOR SHOP AND FIELD PREQUALIFIED WELD JOINTS AND WELD JOINTS QUALIFIED BY TEST SHALL BE PREPARED FOR REVIEW PRIOR TO FABRICATION. ALL WELDING PROCEDURE ITEMS SUCH AS BASE METALS, WELDING PROCESSES, FILLER METALS AND JOINT DETAILS THAT MEET THE REQUIREMENTS OF AWS D1.1 SECTION 5.1 SHALL BE CONSIDERED AS PREQUALIFIED. ANY CHANGE OR SUBSTITUTION THAT IS BEYOND THE RANGE OR TOLERANCE OR REQUIREMENTS FOR PREQUALIFICATION SHALL BE QUALIFIED BY TEST PER AWS D1.1 SECTION 5 PART B. QUALIFICATION TESTING IS REQUIRED WHEN THE DEPTH OF A PARTIAL PENETRATION OR COMPLETE PENETRATION WELD IS 2" OR GREATER.
18. FOR NONDESTRUCTIVE TESTING OF WELDED CONNECTIONS EXCLUDING PRIMARY MEMBERS OF MOMENT RESISTING FRAMES:
    A) WELDED CONNECTIONS SHALL BE TESTED BY NONDESTRUCTIVE METHODS FOR COMPLIANCE WITH AISC J2, AND JOB SPECIFICATIONS. ULTRASONIC TESTING SHALL BE IN ACCORDANCE WITH AWS D1.1, ASTM E164 AND ASME SECTION V. RADIOGRAPHY SHALL BE IN ACCORDANCE WITH AWS D1.1, ASTM E94 AND E99, AND ASME SECTION V. THIS TESTING SHALL BE PART OF THE SPECIAL INSPECTION REQUIREMENTS OF IBC SECTION 1704.3 PERFORMED BY AN APPROVED INDEPENDENT TESTING LABORATORY AS FOLLOWS:
        1. BASE METAL THICKER THAN 1 INCH WHEN SUBJECT TO THROUGH THICKNESS WELD SHRINKAGE STRAINS.
        2. ALL COMPLETE JOINT PENETRATION GROOVE OR BUTT WELDS.
        3. ALL PARTIAL JOINT PENETRATION GROOVE WELDS WHEN USED IN COLUMN SPLICES.
    B) ANY MATERIAL DISCONTINUITIES SHALL BE ACCEPTED OR REJECTED ON THE BASIS OF DEFECT RATING IN ACCORDANCE WITH THE (LARGER REFLECTOR) CRITERIA OF AISC J2.
19. TENSION ROD REPLACEMENTS SHALL BE CAST DUCTILE IRON RODS GRADE 80-55-06 AND CONFORM TO ASTM A-536.

DEMOLITION NOTES

100SN008-1

1. THESE NOTES ALSO APPLY TO CONDITIONS WHERE PARTIAL OR SELECTIVE DEMOLITION IS REQUIRED AS PART OF THE WORK.
2. SAFETY NOTES:
    A. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COMPLY WITH THE PERTINENT SECTIONS, AS THEY APPLY TO THIS PROJECT, OF THE "CONSTRUCTION SAFETY ORDERS" ISSUED BY THE STATE OF CALIFORNIA LATEST EDITION AND ALL OSHA REQUIREMENTS.
    B. THE ARCHITECT, STRUCTURAL ENGINEER AND OWNER DO NOT ACCEPT ANY RESPONSIBILITY FOR THE CONTRACTOR'S FAILURE TO COMPLY WITH THESE REQUIREMENTS.
    C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE DESIGN AND CONSTRUCTION OF ALL FORMWORK, BRACING, AND SHORING REQUIRED. SHORING INDICATIONS (LOCATION, DIRECTION, DURATION, ETC.) ARE ONLY SHOWN ON THE STRUCTURAL DRWGS WHEN REQUIRED TO IMPLEMENT THE DESIGN INTENT OF THE FINAL WORK PRODUCT. DETERMINATION WHETHER SHORING IS REQUIRED FOR TEMPORARY OR INTERMEDIATE CONDITIONS DURING CONSTRUCTION IS WHOLLY THE RESPONSIBILITY OF THE CONTRACTOR.
3. SHORE OR BRACE BEAMS, COLUMNS AND WALLS AS REQUIRED TO MAINTAIN THE STABLE INTEGRITY OF THE EXISTING STRUCTURE PRIOR TO DEMOLITION. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DESIGN AND PROVIDE COMPETENT SHORING AND BRACING FOR ALL LOADS IMPOSED DURING AND AFTER DEMOLITION THROUGH COMPLETION OF NEW CONSTRUCTION.
4. CONTRACTOR IS TO PREPARE A DEMOLITION AND SHORING PLAN DEPICTING LOCATIONS AND METHOD OF SHORING AND DEMOLITION SEQUENCING. ALL DIMENSIONS GIVEN TO EXISTING STRUCTURE ARE APPROXIMATE. VERIFY BY FIELD MEASUREMENTS THE DIMENSIONS OF THE EXISTING STRUCTURE. WHERE ACTUAL CONDITIONS DEVIATE FROM DETAILS SHOWN ON THE DRAWINGS, NOTIFY THE STRUCTURAL ENGINEER FOR INSTRUCTIONS BEFORE PROCEEDING WITH WORK.
5. ANY CONFLICTING ITEMS NOT SPECIFICALLY SHOWN (SUCH AS UNDERGROUND PIPES, CONDUITS, ETC.) SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO FURTHER DEMOLITION.
6. DEMOLITION AND REMOVAL OF EXISTING CONSTRUCTION SHALL BE MADE IN SUCH A MANNER AS TO MINIMIZE OR AVOID DAMAGE TO ADJACENT CONSTRUCTION.
7. A COMPLETE SET OF ORIGINAL DRAWINGS OF THE EXISTING STRUCTURE IS TO BE KEPT ON THE JOBSITE. CONTRACTOR SHALL OBTAIN THESE DRAWINGS FROM THE OWNER WHERE AVAILABLE.
8. EXTENT OF DEMOLITION IS TO BE AS INDICATED ON PLANS, SECTIONS AND DETAILS. VERIFY STORAGE AND REUSE OF EXISTING MATERIALS WITH ARCHITECTURAL DRAWINGS. DEMOLITION IS TO INCLUDE REMOVAL AND DISPOSAL OF DEMOLISHED CONSTRUCTION. SEE SPECIFICATIONS FOR ADDITIONAL DEMOLITION INSTRUCTIONS.
9. CONTRACTOR SHALL RETAIN THE SERVICES OF A CIVIL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA TO PROVIDE SELECTIVE DEMOLITION AND REPLACEMENT OF EXISTING CONSTRUCTION IN AREAS NECESSARY FOR CLEARANCE OF PILE DRIVING EQUIPMENT. PLAN SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW.
10. ALL MATERIALS REQUIRING STORAGE SHALL BE PROTECTED TO PREVENT DEGRADATION.

HISTORIC REHABILITATION NOTES:

1. THE BRIDGE SHALL BE REHABILITATED PER "STANDARDS FOR REHABILITATION AND GUIDELINES FOR REHABILITATING HISTORIC BUILDINGS" AS PUBLISHED BY THE SECRETARY OF THE INTERIOR.
2. CARE SHALL BE PROVIDED TO PROTECT HISTORIC FEATURES PRIOR TO AND DURING CONSTRUCTION.
3. MATERIAL DEGRADATION OR DETERIORATION THAT IS NOT NOTED IN THE DRAWINGS AND IS ENCOUNTERED IN CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE STATE'S REPRESENTATIVE. DETERIORATED MATERIALS SHALL BE REPAIRED AND/OR REPLACED. WHERE POSSIBLE, REPLACED AND/OR REPAIRED PORTIONS SHALL BE INSTALLED TO MATCH THE EXISTING CHARACTERISTICS OF THE STRUCTURE.

REINFORCEMENT LAP SPLICE SCHEDULE (ALL LENGTHS SHOWN ARE IN INCHES.) 3000SN002-1										ACI 318 CBC/IBC
FC' = 4000 PSI CONC (EPOXY COATED)										
SPLICE CLASS	REINF LOCATION	#3	#4	#5	#6	#7	#8	#9	#10	#11
B	TOP	24	48	60	72	105	120	135	150	165
	OTHER	19	37	46	56	81	93	104	116	127
FC' = 3500 PSI CONC (EPOXY COATED)										
SPLICE CLASS	REINF LOCATION	#3	#4	#5	#6	#7	#8	#9	#10	#11
B	TOP	20	52	64	77	113	129	145	161	177
	OTHER	14	40	50	60	87	99	112	124	136

NOTES:
1. SCHEDULE APPLIES TO NORMAL WEIGHT CONCRETE WITH EPOXY COATED, GRADE 60 REINFORCING STEEL FOR #4 BARS AND LARGER (VALUES FOR #3 BARS BASED ON GRADE 40).
2. TOP REINFORCEMENT IS HORIZONTAL REINFORCEMENT LOCATED SUCH THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE.
3. WHEN LIGHTWEIGHT CONCRETE IS USED, MULTIPLY LAP LENGTHS BY 1.30.
4. WHERE CLEAR SPACING OF BARS BEING SPLICED IS LESS THAN 2 BAR DIA. OR WHERE CLEAR COVER OF BARS BEING SPLICED IS LESS THAN 1 BAR DIA., MULTIPLY LAP LENGTHS BY 1.50, UNO.
5. WHERE NOTES #3 AND #4 OCCUR, MULTIPLY LAP LENGTHS BY 2.00, UNO.
6. WHERE CLASS A LAP SPLICE IS NOTED IN DETAIL, DIVIDE LENGTHS ABOVE BY 1.30.
7. WHERE CONCRETE COVER IS GREATER THAN 3 BAR DIA. AND CLEAR SPACING IS GREATER THAN 6 BAR DIA., LAP SPLICE LENGTHS CAN BE MULTIPLIED BY 0.8.

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ACCESSIBILITY SECTION
CERTIFICATION # 18-005

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SOUTH YUBA RIVER STATE PARK
HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

GENERAL NOTES

DRAWING NO. 30419.004

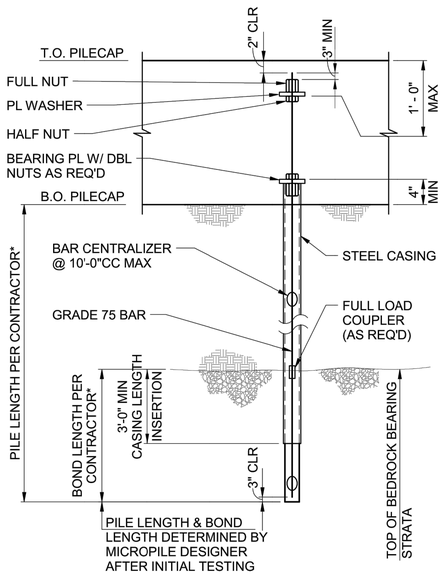
SHEET NO. S0.02

004 OF 030

CD'S

GENERAL NOTES

APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE



* ESTIMATED BOND LENGTH IS 6.25 FEET BASED ON ASSUMED ULTIMATE BOND STRESS PROVIDED IN SOILS REPORT. ACTUAL BOND LENGTH TO BE DETERMINED BY CONTRACTOR AND VERIFIED BY LOAD TESTING. ACTUAL BOND LENGTH SHALL NOT BE LESS THAN 5 FEET.

NOTES:

1. DESIGN LOAD CAPACITY:
   D = DEAD LOAD
   L = LIVE LOAD
   H = SOIL LOAD

SIZE	D	D+H+L	D+H+E
7"Ø	80K	165K	-55K

LOADS GIVEN IN KIPS, 1K= 1000LBS. POSITIVE VALUES ARE DOWNWARD, NEGATIVE VALUES ARE UPLIFT. MINIMUM FACTOR OF SAFETY=2.0 (D+L) SEE GEOTECHNICAL REPORT FOR ESTIMATED ULTIMATE BOND STRESS

2. MATERIAL SPECIFICATIONS:
   A) GROUT - A NEAT MIX OF PORTLAND CEMENT (TYPE I/II) CONFORMING TO ASTM C150 WITH A WATER CEMENT RATIO OF APPROXIMATELY 0.35. THE MINIMUM 28 DAY COMPRESSIVE STRENGTH OF THE GROUT SHALL BE 5,000 PSI.
   B) REINFORCING BAR - THE REINFORCING BAR SHALL BE GRADE 75 DYWIDAG THREADBAR (OR EQUIVALENT) CONFORMING TO ASTM A-615. LENGTH OF COUPLED BAR SECTIONS SHALL BE DETERMINED BASED ON THE OVERHEAD CLEARANCE AVAILABLE AT EACH PILE LOCATION.
   C) CASING - THE STEEL CASING SHALL BE 0.5" MINIMUM WALL THICKNESS - 80 KSI YIELD STRENGTH MANUFACTURED TO API N-80 SPECIFICATIONS.
   D) PLATE - STEEL PLATE SHALL CONFORM TO ASTM A572 GRADE 50.
3. RECOMMENDED MICROPILE INSTALLATION PROCEDURE:
   A) ADVANCE OUTSIDE DIAMETER CASING TO FULL PILE DEPTH UTILIZING ROTARY DRILLING TECHNIQUES.
   B) PLACE REINFORCING TREAD BAR WITH CENTRALIZERS.
   C) TREMIE CASING FULL WITH NEAT CEMENT GROUT.
   D) EXTRACT CASING OUT OF THE PILE BOND LENGTH WHILE PLACING ADDITIONAL GROUT. PRESSURE SHALL BE APPLIED TO THE BOND LENGTH GROUT A MINIMUM OF 2 TIMES DURING CASING EXTRACTION TO A MINIMUM PRESSURE OF 75 PSI. THIS PRESSURE MAY BE APPLIED THROUGH THE DRILL.
   E) UPON COMPLETION OF PRESSURE GROUTING, REINSERT THE CASING 3' TO 4' MINIMUM INTO THE TOP OF THE BOND LENGTH AS SHOWN ON THE PILE DESIGN DETAILS. APPLY PRESSURE TO THE GROUT ONE FINAL TIME AFTER CASING REINSERTION.
   F) THE TOP PIECE OF CASING SHALL BE PREFABRICATED WITH THE CONNECTION SHEAR RINGS WHERE OCCURS. REMOVE UNSOUND GROUT FROM THE TOP OF THE PILE CASING AND PLACE THE BEARING PLATE AS REQUIRED BY THE CONNECTION DETAILS. CAST PILE TOP INTO THE FOOTING CONCRETE.
   G) THE QUALITY OF THE GROUT SHALL BE MONITORED BY COLLECTING GROUT CUBES EACH DAY FOR LATER COMPRESSION TESTING.
   H) CONSISTENCY OF PILE INSTALLATION SHALL BE MONITORED AND RECORDED IN THE MICROPILE INSTALLATION LOG FORM. MONITORED AND RECORDED DATA SHALL INCLUDE TOTAL PILE DEPTH, GROUT PRESSURES AND QUANTITIES, SOILS ENCOUNTERED DURING INSTALLATION, AND ANY OBSTRUCTIONS OR IRREGULARITIES.
   I) CONSTRUCTION TOLERANCE ON PLACEMENT OF PILES IS 3 INCHES HORIZONTALLY IN ANY DIRECTION AND A VERTICAL ALIGNMENT OF NO MORE THAN 2% OUT OF PLUMB.
4. DESIGN OF PILES TO CONFORM WITH SOILS REPORT # S9030-05-41 BY GEOCON CONSULTANTS DATED MAY 2015.
5. INSTALLATION TO BE PERFORMED BY A QUALIFIED CONTRACTOR WITH A MINIMUM OF 5 YEARS OF EXPERIENCE WITH THIS TYPE OF FOUNDATION.
6. INSTALLATION RECORDS AND INITIAL TESTING TO BE DONE IN ACCORDANCE WITH SPECIFICATIONS. LOAD TESTS SHALL BE COMPLETED ON PRE-PRODUCTION AND PRODUCTION PILES AS REQUIRED IN THE SOILS REPORT AND CBC 1810.3.3.
7. MINIMUM PILE SPACING 3'-0" CENTER TO CENTER.
8. PROVIDE PILE DESIGN CALCULATIONS FOR APPROVAL SIGNED BY CIVIL ENGINEER LICENSED IN THE STATE OF CALIFORNIA.

MICRO-PILE NOTES

222SD001-12

1 / S0.03

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SOUTH YUBA RIVER STATE PARK
HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

GENERAL NOTES

DRAWING NO.
30419.005

SHEET NO.
S0.03

005 OF 030

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CD'S

TYPICAL DETAILS

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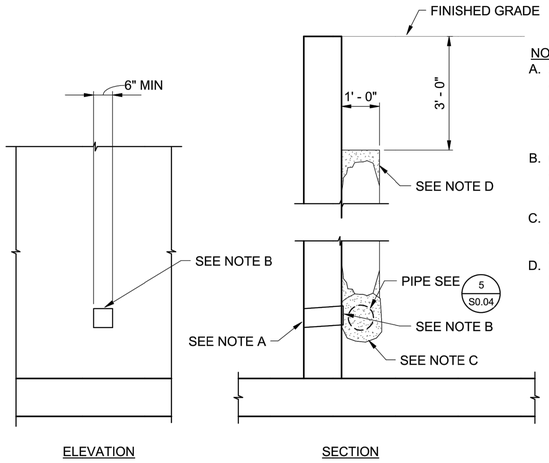
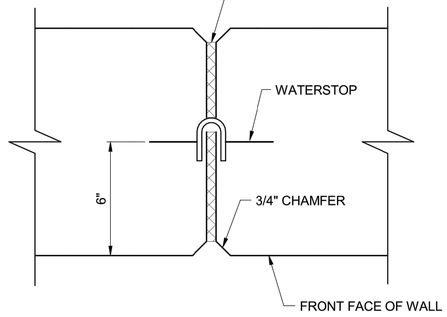


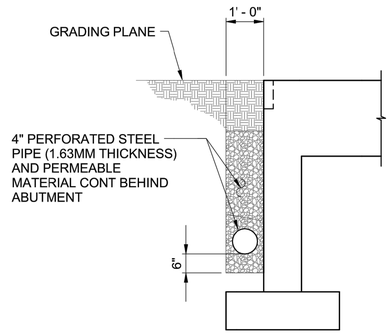
NOTES:

A. 2"Ø DRAINS @ 48"CC MAX, FOR WALLS ADJACENT TO SIDEWALKS OR CURBS. PROVIDE 4" PLASTIC PIPE UNDER THE SIDEWALK TO DISCHARGE THROUGH CURB FACE. EXPOSED WALL DRAINS SHALL BE LOCATED 3"± ABOVE FINISHED GRADE.
B. 6" MIN SQUARE ALUMINUM OR GALVANIZED STEEL WIRE 1/4" MESH HARDWARE CLOTH (22GA MIN WIRE DIAMETER). ANCHOR FIRMLY TO BACKFACE.
C. 1 CUBIC FOOT PERVIOUS BACKFILL MATERIAL IN A NONWOVEN FILTER FABRIC, SECURELY TIED.
D. PERVIOUS BACKFILL MATERIAL CONTINUOUS BEHIND RETAINING WALL OR ABUTMENT.

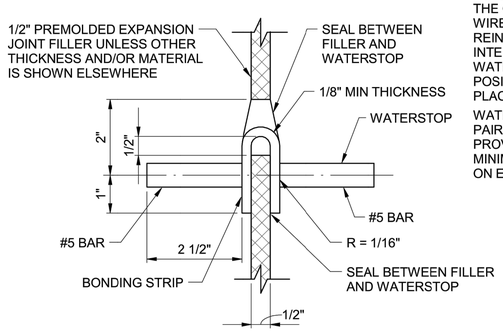
1 S0.04 WEEP HOLE AND PERVIOUS BACKFILL

NOTE:
FOR PLACEMENT IN BRIDGE DECKS, SEE JOINT SEAL DETAILS. FOR WALL THICKNESSES LESS THAN 12", USE 1/2 THE WALL THICKNESS





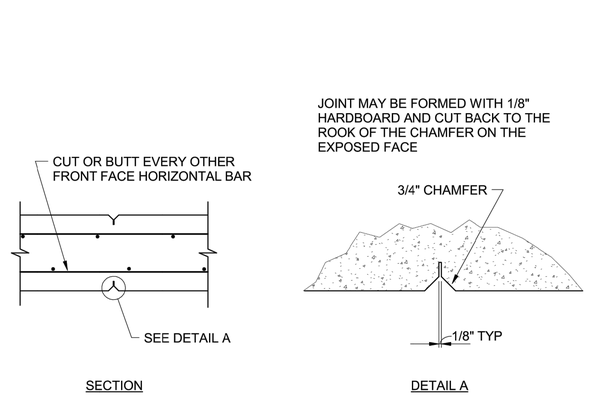
2 S0.04 WEAKENED PLANES



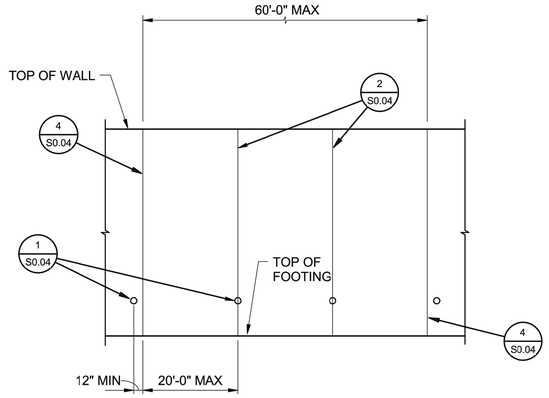
NOTES:

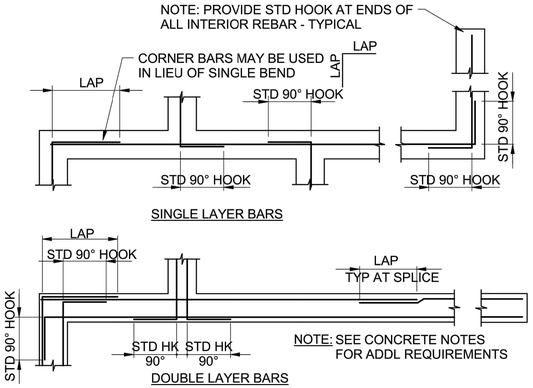
HOLES WILL BE PERMITTED IN THE OUTER 1/2" OF THE WEB FOR WIRE, RINGS, ETC. TIE WEB TO #5 REINFORCING BARS @ 16"CC MAX INTERVALS TO SUPPORT THE WATERSTOP IN PROPER POSITION DURING CONCRETE PLACEMENT.

WATERSTOP TO HAVE 5 OR MORE PAIRS OF RAISED RIBS TO PROVIDE 0.10 SQUARE INCHES MINIMUM CROSS-SECTION AREA ON EA HALF OF THE WATERSTOP.

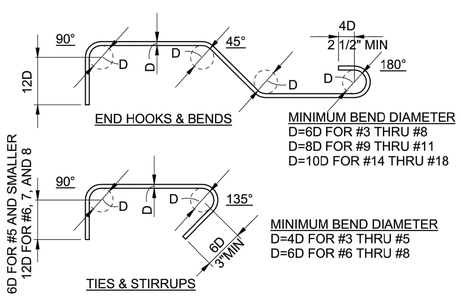


3 S0.04 WALL EXPANSION JOINTS AND WEAKENED PLANES



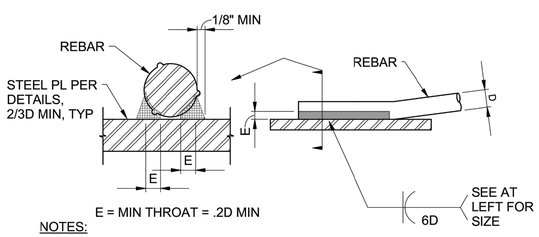


4 S0.04 WALL EXPANSION JOINT



NOTE: ALL HOOKS SHALL BE 90° OR 180° STANDARD HOOKS UNLESS OTHERWISE SHOWN OR NOTED.

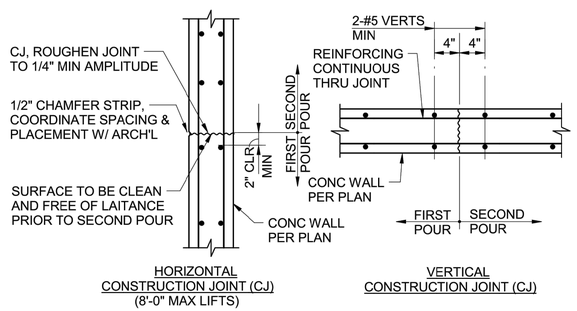
5 S0.04 DRAIN PIPE AND PERMEABLE MATERIAL



NOTES:

1. REINFORCING TO BE WELDED, EXCEPT ASTM A706, SHALL CONFORM TO THE MATERIAL PROPERTY REQUIREMENTS OF ANSI/AWS D1.4, NEWEST EDITION.
2. ALL PREHEATING AND WELDING SHALL BE DONE IN ACCORDANCE WITH ANSI/AWS D1.4, NEWEST EDITION.
3. ALL WELDING SHALL BE CONTINUOUSLY INSPECTED BY A QUALIFIED LABORATORY.

6 S0.04 WATERSTOP



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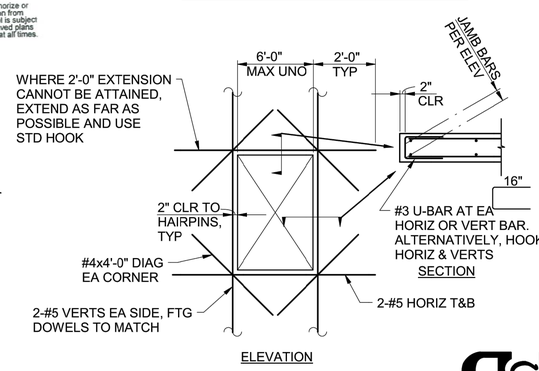
7 S0.04 CORNER REINFORCING AT CONCRETE FOOTINGS



8 S0.04 STANDARD REBAR HOOKS AND BENDS

9 S0.04 REBAR WELDING

300SD002-32

10 S0.04 CONCRETE WALL JOINTS

320SD003-12

11 S0.04 OPENING REINFORCING IN CONCRETE WALLS

320SD007-32

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HISTORIC COVERED BRIDGE REHABILITATION AND RESTORATION

TYPICAL DETAILS

DRAWING NO.
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006 OF 030

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